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Assessment on maximum magnitude of natural and triggered earthquake when water is impounded in the mining pit: A case study in Nui Nho quarry, Vietnam based on gravity and magnetic data

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Abstract. The paper presents the assessment of the maximum magnitude of natural and triggered earthquakes when water is impounded to 130 m in the mining pit for the landscaped lake in the Nui Nho quarry in Di An commune, Binh Duong province, Vietnam. The maximum magnitudes of the natural and triggered earthquakes were estimated by using artificial neural network and source segmentation methods, respectively. Gravity method was used to estimate fault parameters including strike, dip and rake angles. As well as, these fault parameters are considered as input information to compute the influence of Nui Nho reservoir impoundment by using the concept of fault stability Coulomb stress in order to assess risk of triggered earthquake occurrence. The results show that the Nui Nho quarry is within the sphere of influence of second order Dong Nai River fault zone; the magnitude of natural

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earthquake that can occur in this area is 5.0; the triggered earthquake source of Nui Nho lake has the length of 4.8 km and the width of 1.8 km; the maximum magnitude of triggered earthquake can only be less than or equal to 3.2; the reservoir with depth of 130 m can cause direct stress on the lake bottom with the maximum value of 47.295 kPa and Coulomb stress field with the value of 4.338 kPa. The region of positive Coulomb stress shows the influence of reservoir impoundment indicating the region of risk of triggered earthquakes occurrence.

1. Introduction

Scientists have found that artificial lakes with considerable depths (e.g. over 90 m) can possibly cause earthquakes [*Gupta*, 2002, 2011; *Gupta et al.*, 1972; *Kalpna and Chander*, 2000; *Kalpna and Gupta*, 2008; *Kalpna et al.*, 2016]. The weight of water mass buckles the basement rock of lake bottom, causing subsidence (the subsidence of lake bottom can be up to $1.5 \div 2$ m), which is the reason for triggering the reactivation of faults in the region, increasing the horizontal permeability, especially in the fracture zones. The increase of pore pressure in rock increases the infiltration, thus reducing the friction of existing conformities or failure surfaces. As a result, it decreases the firmness and changes the elastic coefficient of rock, causes the fluctuation of preceding tension force, leading to the displacement of rock mass towards the tectonic failure surface [*Gupta*, 2002, 2011; *Gupta et al.*, 1972; *Hagan et al.*, 1996; *Hojjat and Panakkat*, 2009; *Kalpna and Chander*, 2000; *Kalpna and Gupta*, 2008; *Kalpna et al.*, 2016].

Triggered earthquake does not occur in all the lakes. It only occurs in the lakes with favorable geological and tectonic conditions: they are active zones, the tectonic stress in rock reaches the limit, the faults hydrologically related to the lakes are large enough. In case the stress reaches the limit, the incremental stress caused by reservoir loading can play a role as the promoting mechanism although it is very small. According to statistics [Hagan et al., 1996], the percentage of lakes with a depth of 90 \div 120 m in which triggered earthquake can occur is 6% (5/78 lakes); that with a depth of $120 \div 150$ m is 17% (5/29 lakes) and that with a depth of $150 \div 250$ m is 26% (5/19 lakes). Triggered earthquakes have also occurred at some hydropower reservoirs in Vietnam, most notably in Hoa

Binh, Song Tranh 2 and Son La [Pupkov et al., 2012; Son, 2012; Trong, 2012; Telesca et al., 2016; Tuan et al., 2017]. On 23 May 1989 an earthquake with M = 4.9 occurred in Hoa Binh area after impounding water in the hydropower reservoir to the maximum elevation (90 m). Shortly after the Song Tranh 2 hydropower reservoir was impounded (up to 96 m height), triggered earthquakes occurred, with the largest magnitude M = 4.7 on 15 November 2012. The strongest triggered earthquake at Son La hydropower reservoir has a magnitude of 4.3, recorded on 19 July 2014. The geological structures of Hoa Binh, Song Tranh 2 and Son La reservoirs are metamorphic rocks (including gneiss, metagabbro, metadiorite, metagranodiorite, metaplagiogranite), granite, andesite or strongly cataclastic, fractured compact limestone.

The Nui Nho quarry in Di An commune, Binh Duong province is currently exploited at -100 m. In the future, this quarry will be exploited to -130 m and converted into a landscaped lake. Does the water impoundment in a lake of such great depth cause triggered earthquake? The managers and scientists in Binh Duong province claim that the geological and tectonic characteristics of Nui Nho are similar to those of Song Tranh 2, where the triggered earthquakes have occurred

frequently after the water impoundment. Natural earthquakes have not been recorded in Nui Nho quarry; similarly, in Song Tranh 2, there have never been natural earthquakes before the occurrence of triggered earthquakes. That is the question that needs to be answered before the depth of exploitation and conversion of utility are approved. The assessment of the maximum magnitude of natural earthquake plays an important role in calculating earthquake hazard and determining the maximum magnitude of the triggered earthquake if it occurs. The maximum magnitude of the triggered earthquake does not exceed that of natural earthquakes in the study area [Son, 2012; Trieu et al., 2014; Trong et al., 2016; Tuan et al., 2017]. In this paper, a comprehensive study is described which estimates source parameters, maximum magnitude of natural and triggered earthquakes. The authors present in detail the modern approach, using artificial intelligence (artificial neural network) in assessing the maximum magnitude of natural earthquakes in the study area. The gravity method was used to estimate source dimension to calculate the maximum triggered earthquake. Finally, modelling of Coulomb stresses due to reservoir water loading has been used to see the effect of reservoir impoundment in the Nui Nho area.

2. Assessment of Maximum Natural Earthquake (Mn.max) in the Nui Nho Quarry and Adjacent Area

2.1. The Earthquake Data (Earthquake Catalogue)

Xuyen [2004] and Trieu [2010a, 2010b] argued that the earthquake activities in the Nui Nho quarry and adjacent area in particular and in Southern Vietnam in general are not strong compared to in extreme Northwestern Vietnam and the earthquake magnitude here does not exceed 6.0. According to historical records, some earthquakes with M = 5.0 also occurred in this area before the 20th century, for example in 1715 (M = $4.0 \div 5.0$), in 1877 ($M = 5.0 \div 5.5$), 1882 ($M = 5.0 \div$ 5.5). The Institute of Geophysics (Vietnam Academy of Science and Technology) manages the earthquake data in Vietnam and has responsibility for updating annual earthquake data [Trieu, 2010a, 2010b; Xuyen [2004]. Due to the war and the lack of attention to earthquakes in historical records, the earthquake catalogue obtained in the Institute of Geophysics is deficient in data and the accuracy of M determination is also low. The error of M determination accord-

ing to historical records and public surveys is probably not less than 1.0 M unit. Only after 2002 when 7 earthquake observation stations were established in this area, there has been sufficient information about earthquakes with $M \ge 3.5$ [Hung, 2009; Trieu, 2010a, 2010b]. Therefore, in order to have the best earthquake catalogue for maximum natural earthquake forecasting (Mmax) in the Nui Nho quarry and adjacent area, the authors have collected more earthquake data of ISC (http://www.isc.ac.uk/iscbulletin/search/bulletin/ interactive/). CN algorithm [*Peresan et al.*, 2000; *Trieu*, 2010b] is used to analyze, compare and produce the most reliable earthquake catalogue from 1900 to 2017 (Figure 1).

2.2. The Maximum Natural Earthquake (Mn.max) in the Nui Nho Quarry and Adjacent Area

The feedforward neural network with backpropagation algorithm was applied to calculate the maximum magnitude of natural earthquake [*Bhatia*, 2018; *Hagan et al.*, 1996; *Hojjat and Panakkat*, 2009; *Pupkov et al.*, 2012; *Trong et al.*, 2016]. Input data for calculation include: 1) Gradient value of elevation; 2) Gradient value of Bouguer gravity field (Vietnam Geophysical Divison,



2010); 3) Gradient of aeromagnetic anomaly (Vietnam Geophysical Divison, 1995); 4) Gradient value of isostatics [*Trieu*, 2010b]; 5) Gradient of sedimentary crust thickness [*Hung*, 2009]; 6) Gradient of crystalline basement depth [*Hung*, 2009]; and 7) Gradient of the Earth's crust thickness [*Hung*, 2009]. The distribution map of maximum magnitude of natural earthquake (Mn.max) according to our calculation result is shown in Figure 1.

Input parameters are the data that are assessed as directly related to the earthquake magnitude. Standard samples are typical earthquakes with magnitude greater than or equal to 2.5 and with earthquake catalogue from 1900 to 2017. The analysis and processing of input data for calculating and forecasting maximum natural earthquake (Mtn.max) are conducted as follows: Firstly, the study area is divided into the square grid with side of 0.5 km; then, the data matrix is established based on the result of analysis of 7 existing data at the grid points. The investigation and selection of neural network are carried out in accordance with the strict process [Trong, 2012, 2019; Trong et al., 2019]. According to the neural network construction algorithm, among 100% of the samples, we take 70% to build network parameters, 15% to check the

rationality and the remaining 15% to recheck the network. For each of the above percentages there is an evaluation R; after synthesizing and averaging we get the result R of neural network. It should be noted that due to the limited number of earthquakes observed, we have to increase the study area to ensure the representativeness of earthquake magnitude. The forecasting result (Mtn.max) presented in Figure 1 shows that:

- In Nui Nho quarry and adjacent area, the magnitude of strongest earthquakes cannot exceed 5.5 and they mainly occur in the sea. This value is not much different from previous studies, for example, *Xuyen* [2004] argued that the magnitude of strongest earthquakes in this area cannot exceed 6.0. Based on NeoDitermenestic, *Trieu* [2010b] also suggested M < 6.0 in this area. The calculation of Gumbel function [*Hung*, 2009] produced the result M = 5.5.
- Although there has been no earthquake so far in Nui Nho quarry, in case of earthquake occurrence in the future, the magnitude (Mtn.max) can be less than 5.0.

3. Assessment of Maximum Triggered Earthquake (Mt.max) in the Nui Nho Quarry

Previous studies have suggested that the Nui Nho quarry is located in Dong Nai River active fault zone. Sai Gon River active fault zone is in the southwest of Nui Nho quarry (Figure 2). Both faults can possibly cause earthquakes [*Hung*, 2009; *Linh et al.*, 2008; *Trieu*, 2010a, 2010b; *Xuyen*, 2004], however, the segment of Dong Nai River active fault zone in Nui Nho quarry is the source of triggered earthquake when water is impounded to 130 m high.

The authors have used empirical formulas on the relationship between source parameters (volume, width, length) and maximum earthquake magnitude by *Wells and Coppersmith* [1994] and *Trieu* [2010b]. Therefore, it is necessary to determine the structural characteristics of sources which can possibly cause triggered earthquake.



Figure 2. Nui Nho quarry located in Dong Nai River active fault zone [*Xuyen*, 2004; *Linh et al.*, 2008; *Hung*, 2009; *Trieu*, 2010a, 2010b] and highly detailed gravity measurement profiles.

3.1. Determination of Earthquake Source Parameters in Nui Nho Quarry

High-precision gravity method was chosen to determine earthquake source parameters in Nui Nho quarry, based on the opinions that rocks are strongly fractured when water penetrates the ground and changes pore pressure, which is the cause of earthquake [*Gupta*, 2002, 2011; *Gupta et al.*, 1972; *Kalpna and Chander*, 2000; *Kalpna and Gupta*, 2008; *Kalpna et al.*, 2016]. Meanwhile, the strongly fractured zone is easily recognized through low value of normalized full gradient GH(x, z) compared to monolithic environment [*Trong et al.*, 2016]. The boundary of this zone is also determined by the maximum horizontal gradient of Bouguer gravity anomaly (Gmax) at different depths [*Trong et al.*, 2016].

Based on the measurements of 3 highly detailed gravity profiles (Figure 2, 1000 measurement points, the scale of 1/50,000), 2 best profiles of cross-sectional analysis are selected to determine the parameters of sources which can cause triggered earthquakes (Figure 3a–Figure 3b). The study results allow determining the structural characteristics of earthquake source [*Trong et al.*, 2019]: width of 2.8 km, depth of 12 km, and dip of 80° for natural earthquake source; width of



1.8 km, depth of 12 km, and dip of 80° for triggered earthquake source (in which rocks are fractured more strongly and penetrated more easily by water when connected to reservoir). These parameters are used in determining maximum triggered earthquake (Mt.max).

3.2. The Maximum Triggered Earthquake in the Nui Nho Quarry

Vietnamese seismologists often use the empirical formulas of *Wells and Coppersmith* [1994] and *Trieu* [2010b] in assessing the values of maximum earthquakes for the regions with inadequate earthquake data. Although these two formulas have different coefficients, they have the same result [*Hung*, 2009; *Trieu*, 2010a, 2010b; *Trong et al.*, 2019]. These two formulas are also used for the Nui Nho quarry. Calculation results show that:

- The maximum natural earthquake (Mn.max) determined according to Wells and Coppersmith [1994], *Trieu* [2010b] has the magnitude of 5.0. This result is consistent with the calculation result based on the approach of artificial neural network.
- The maximum triggered earthquake (Mt.max) in Nui Nho quarry has the magnitude of 3.2.

4. Calculation of Incremental Stress on the Reservoir Bottom and Coulomb Stress Change After Reservoir Impoundment

There are 3 major effects of reservoir loading associated with triggered earthquake: 1) The changes of elastic stress related to water impoundment of reservoir; 2) The increase of pore pressure in water-saturated rocks (due to the reduction of pore volume caused by compression) in the reaction to the increase of elastic stress; and 3) The changes of pore pressure related to water permeability.

4.1. Calculation of Incremental Stress on the Reservoir Bottom After Reservoir Impoundment

To calculate the incremental stress and subsidence of the reservoir bottom, both 2D and 3D problems can be used [Kalpna and Chander, 2000; Kalpna and Gupta, 2008; Kalpna et al., 2016]. In the case of the 3D problem, the reservoir is subdivided into the area elements of $(a) \times (a) \text{ km}^2$ by a set of orthogonal straight lines. After determining the water depth (hi) for each area element, we have the vertical force $Fi = \rho ga^2 hi$ at the center of each area that can replace the water column pressure. With the X'X axis towards the east, the Y'Y axis towards the north and the Z'Z axis downwards, 3 normal stress components and 1 tangential stress component at any point P are calculated according to the following formulas:

$$\sigma_r = \frac{F}{2\pi} \left[\frac{1 - 2\gamma}{r^2} \left(1 - \frac{z}{R} \right) - \frac{3r^2 z}{R^5} \right]$$
$$\sigma_z = \frac{3F}{2\pi} \frac{z^3}{R^5}$$

$$\sigma_{\theta} = \frac{F}{2\pi} (1 - 2\gamma) \left(-\frac{1}{r^2} + \frac{z}{r^2 R} + \frac{z}{R^3} \right)$$

$$\tau_{xz} = -\frac{3F}{2\pi} \frac{rz^2}{R^5}$$

where: ν is Poisson's ratio; $R = \sqrt{x^2 + y^2 + z^2}$ – the distance from the origin to the point P(x, y, z) and its projection given by $r = \sqrt{x^2 + y^2}$.

To add the distribution of F forces, converting to Cartesian coordinate system according to the correlation:

$$\sigma_{x} = \tau_{xy} = \\ \sigma_{\theta} \sin^{2} \theta + \sigma_{r} \cos^{2} \theta \quad (\sigma_{r} - \sigma_{\theta}) \sin \theta \cos \theta \\ \sigma_{y} = \tau_{xy} = \\ \sigma_{r} \sin^{2} \theta + \sigma_{\theta} \cos^{2} \theta \quad (\sigma_{r} - \sigma_{\theta}) \sin \theta \cos \theta \\ \sigma_{z} = \sigma_{z} \quad \tau_{yz} = \tau_{rz} \sin \theta$$

where: the azimuth $\theta = \arctan(y/x)$ is calculated from the east to the north (counterclockwise). Stress is considered as the result of total loading on a point collected by taking the total distribution of all F forces for 6 stress components: σ_x , σ_y , σ_z , τ_{xy} , τ_{yz} , τ_{zx} . From these parameters, we select the downward normal stress σ_z and the maximum tangential stress $\tau_{max} =$ $(\sigma_1 - \sigma_3)$. In addition, under the pressure of reservoir loading, the vertical subsidence $\Delta d(m)$ due to the effect of total force F is calculated by the formula:

$$\Delta d = \frac{F}{2\pi E} \left[\frac{(1+v)Z^2}{R^3} + 2\frac{(1-v^2)}{R} \right]$$

where: E is Young's modulus; R is the distance of point P from the origin. The subsidence is caused by all the point forces and the total subsidence d at point P is the result of reservoir loading.

The calculation of incremental stress on the bot-

tom of the Nui Nho reservoir after water impoundment is carried out at the depths of 2 km, 4 km, 6 km, and 8 km (thehypocenter of the triggered earthquake is usually located at the depth of 2 km to 8 km). The results of stress calculation at different depths with water columns of 100 m and 130 m are presented in (Figure 4a–Figure 4d), and the maximum value of this incremental stress is shown in Table 1.

4.2. Calculation of Coulomb Stress Caused by the Effect of the Water Column on the Bottom of Nui Nho Reservoir

According to *Bell and Nur* [1978], the change of Coulomb stress(ΔS) caused by water impoundment in the reservoir is determined as follows: $\Delta S = \Delta \tau - \mu (\Delta \sigma_n - \Delta P)$, where $\Delta \tau$ and $\Delta \sigma_n$ correspond to the changes of tangential stress and normal stress which are caused by the reservoir loading on the fault surface, ΔP is the change of pore pressure, and μ is the coefficient of friction. The increase of $\Delta \tau$ and the decrease of $\Delta \sigma_n$ mean that ΔS has a positive value, which will stimulate the fault activity and vice versa. The role of pore pressure always promotes the fault activity due to the lubrication on the fault surface and decreases the



Figure 4. Stress according to the vertical component: a - at 2 km; b - at 4 km; c - at 6 km; d - at 8 km depth; (reservoir depth of 130 m).

able 1. The Maximum Valu epth Scenario of reservoir depth	e of Incremer Maximu	ital Stress ' m value o	With Different S f incremental st (kPa = 0.01 b	cenarios of Reservoir ress at different depth ar)
	2 km	4 km	6 km	8 km
100 m	36.381	9.264	4.131	2.327
130 m	47.295	12.043	5.371	3.025

tangential stress component $\Delta \tau$. Based on the above theoretical basis, the incremental stress and Coulomb stress caused by the reservoir loading have also been modelled to see the effect of reservoir impoundment in the Nui Nho area. The reservoir is subdivided into small blocks, the parameters of length, width and depth at each block are determined. The fault parameters consisting of the strike, dip, and rake are included to calculate the change of stress field. In the study area, Dong Nai River fault near the lake is considered active and the parameters of this fault (strike = 140° , dip = 80° , rake $= 180^{\circ}$) are included in the calculation. The study area is gridded into $0.0018^{\circ} \times 0.0018^{\circ}$; Poisson's ratio $\nu = 0.25$; Skempton's coefficient B = 0.7; coefficient of friction $\mu = 0.65$.

Components of Coulomb stress field are calculated with reservoir depths of 100 m and 130 m at the depths of 2 km, 4 km, 6 km, and 8 km, respectively (Table 2, Figure 5a–Figure 5d). The calculation results show that the areas with positive value of Coulomb stress ΔS are at risk of triggered earthquake when water is fully impounded in the reservoir. On this basis, it is possible to delineate the areas at risk of reservoir-triggered earthquake.

Direct stress and Coulomb stress caused by the high-



Figure 5. Coulomb stress field caused by reservoir loading: a - at 2 km; b - at 4 km; c - at 6 km; d - at 8 km depth (reservoir depth of 130 m).

Table 2. The Maximum Valu Different Scenarios of Reservoir	e of Could Depth	omb Stres	s Caused by R	teservoir Loading With
The scenario of reservoir depth	Maxim	ium value	of incrementa (kPa	l stress at different dept)
	2 km	4 km	6 km	8 km
100 m	3.336	0.868	0.389	0.219
130 m	4.338	1.129	0.505	0.285

est water column of 130 m have the values of 47.295 kPa and 4.338 kPa, respectively. Compared to the breaking stress of rock in the earthquake (from several hundred bars to several thousand bars), the calculation value is very small, only about 1%. It plays a role as the promoting mechanism and only makes sense when natural stress reaches its limit.

5. Conclusion

Based on the research results, some following conclusions can be drawn:

- 1. Dong Nai River fault is an accompanying branch of the main Sai Gon River fault. The fault is assessed as active in Late Miocene–Pleistocene, in Holocene and recent time, according to the Eman measurement data. The Nui Nho quarry is located within the sphere of influence of second order active Dong Nai River fault.
- 2. The triggered earthquake source in the Nui Nho lake has a length of 4.8 km and a width of 1.8 km. The maximum magnitude of natural earthquakes in this source is 5.0 and that of the triggered earthquake can only be less than or equal to 3.2.

3. The water column of 130 m can cause direct stress on the lake bottom with the maximum value of 47.295 kPa and Coulomb stress with the value of 4.338 kPa. The region of positive Coulomb stress indicating the region of risk of triggered earthquakes occurrence. Compared to the breaking stress of rock in the earthquake, the calculation value is very small, only about 1%. It plays a role as the promoting mechanism and only makes sense when natural stress reaches its limit.

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